



Geodetic Reference Systems

Huaan Fan

Royal Institute of Technology (KTH)
Stockholm, Sweden

GEOWEB training course on Modern Geodetic Concepts
October 16-20, 2017. University of Mostar, Bosnia & Herzegovina

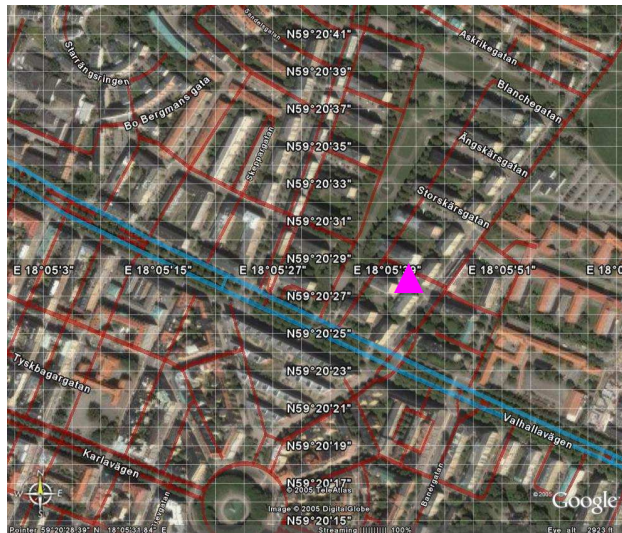


GRS overview

- Why? What is new?
- Astrogeodetic triangulation (2D)
- Height systems from levelling (1D)
- 3D geocentric reference systems
- Swedish geodetic infrastructure



Stockholm in Google Earth (2005)



Satellite images are in **WGS 84**

Street lines (**red** or **blue** lines) are in Stockholm city's **local coordinate system**



An old subject becomes a new topic

- High (10^3) accuracies of space geodetic methods (GNSS, VLBI, SLR, DORIS)
 - individual measurements more accurate than the system
 - astro-geodynamic effects become significant
- Needs for 3D reference systems, instead of 2+1D
- Globalization of positioning and mapping
- Widely use of geospatial databases and GIS
- Transition period for both old and new GRS
- How to make levelling effective or replace it?



Reference system vs reference frame

- **Reference system:** theoretical definition of the origin, scale, orientation of the 3 coordinate axes, together with related mathematical and physical assumptions
- **Reference frame:** a network of ground points or celestial bodies, at which the coordinates and velocities in a certain reference system have been explicitly specified, as a realization of the theoretical reference system

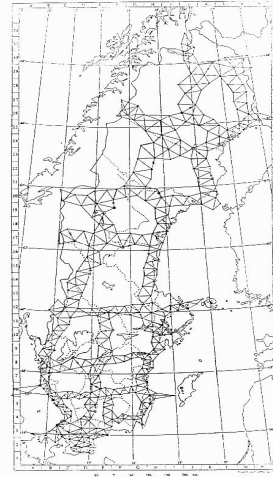
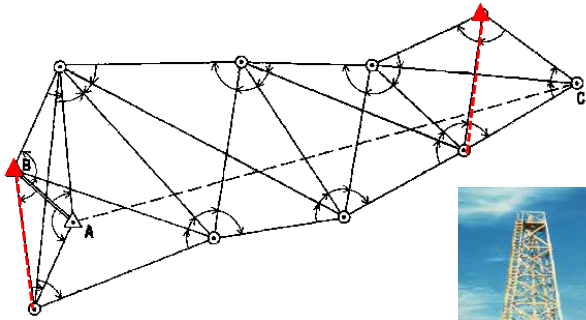


GRS overview

- Why? What is new?
- **Astrogeodetic triangulation** (2D)
- Height systems from levelling (1D)
- 3D geocentric reference systems
- Geodetic infrastructure in Sweden



Astrogeodetic triangulation



General procedures

- Select a reference ellipsoid and define a *datum*
- Design a network of triangles
- Astronomical observations (*a fewer*)
- Geodetic measurements (angles, distances etc)
- Reduction of ground measurements to the ellipsoid
- Least squares adjustment on the ellipsoid
- Define an official 2D coordinate system
- Re-estimation of new ellipsoidal parameters



Reduction of astronomical coordinates

$$\begin{aligned}\phi &= \Phi - \xi \\ \lambda &= \Lambda - \eta / \cos \phi\end{aligned}$$

$$\alpha_{ab} = A_{ab} - \Delta A_1 - \Delta A_2$$

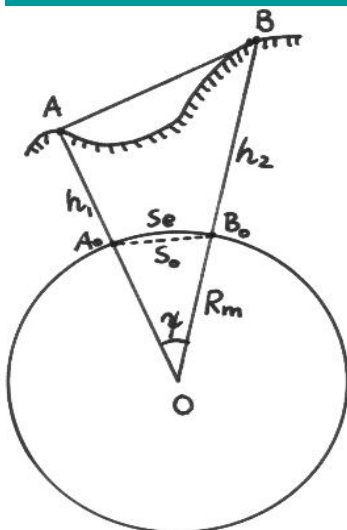
$$\left. \begin{aligned}\Delta A_1 &= \eta \operatorname{tg} \phi = (\Lambda - \lambda) \sin \phi \\ \Delta A_2 &= (\xi \sin \alpha_{ab} - \eta \cos \alpha_{ab}) \cot z_{ab}\end{aligned} \right\}$$

When $z_{ab} \sim 90^\circ$, we get the **Laplace condition**:

$$A_{ab} - \alpha_{ab} = (\Lambda - \lambda) \sin \phi$$



Reduction of slope distances



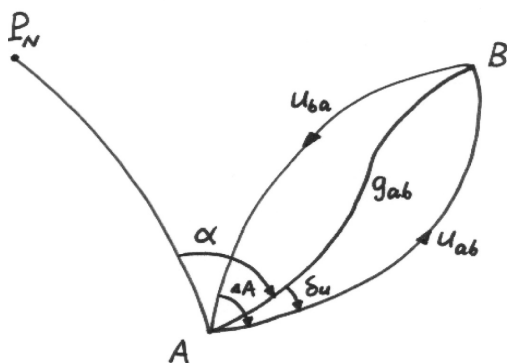
$$s_0 = \sqrt{\frac{s^2 - (h_2 - h_1)^2}{\left(1 + \frac{h_1}{R_m}\right) \left(1 + \frac{h_2}{R_m}\right)}}$$

$$R_m = \frac{a\sqrt{1-e^2}}{W^2} = \frac{a\sqrt{1-e^2}}{1-e^2 \sin^2 \phi_m}$$

$$s_e = 2R_m \cdot \arcsin \left(\frac{s_0}{2R_m} \right)$$



Reduction of ground directions (angles)



- Reduction due to deflection of the vertical

$$A_{ab} - \alpha_{ab} = (\Lambda - \lambda) \sin \phi$$

- Reduction due to discrepancy between normal section and geodesic

$$\delta_u \approx \Delta A/3 \approx \frac{e^2 \cdot s^2}{12 N^2} \cos^2 \phi \sin 2\alpha$$

- Reduction due to height of the object above the ellipsoid

$$\delta_h \approx -\frac{h}{2N} e'^2 \cos^2 \phi \sin 2\alpha$$



Least squares adjustment on the ellipsoid

- **Unknown parameters to be determined:** geodetic latitudes and longitudes of unknown triangulation points
- **Observations:** distances, azimuths, angles/directions
- **Observation equation:** an explicit function of the unknown parameters for each observation
- **If an observation equation is non-linear:** the observation equation must be linearized using approximate geodetic latitudes/longitudes



Observation equation of a distance

$$s_{ij} - \varepsilon_{ij} = s(\phi_i^0 + \delta\phi_i, \lambda_i^0 + \delta\lambda_i, \phi_j^0 + \delta\phi_j, \lambda_j^0 + \delta\lambda_j)$$

$$\approx s_{ij}^0 + \frac{\partial s}{\partial \phi_i} \cdot \delta\phi_i + \frac{\partial s}{\partial \lambda_i} \cdot \delta\lambda_i + \frac{\partial s}{\partial \phi_j} \cdot \delta\phi_j + \frac{\partial s}{\partial \lambda_j} \cdot \delta\lambda_j$$

$$l_{ij} - \varepsilon_{ij} = a_i \cdot \delta\phi_i + b_i \cdot \delta\lambda_i + a_j \cdot \delta\phi_j + b_j \cdot \delta\lambda_j$$

$$\begin{aligned} a_i &= -M_i^0 \cos \alpha_{ij}^0 \\ b_i &= N_j^0 \cos \phi_j^0 \sin \alpha_{ji}^0 \\ a_j &= -M_j^0 \cos \alpha_{ji}^0 \\ b_j &= -N_j^0 \cos \phi_j^0 \sin \alpha_{ji}^0 \\ l_{ij} &= s_{ij} - s_{ij}^0 \end{aligned}$$

For linearization:

$$\begin{aligned} \phi_i &= \phi_i^0 + \delta\phi_i \\ \lambda_i &= \lambda_i^0 + \delta\lambda_i \\ \phi_j &= \phi_j^0 + \delta\phi_j \\ \lambda_j &= \lambda_j^0 + \delta\lambda_j \end{aligned} \quad \begin{aligned} s_{ij}^0 &= s(\phi_i^0, \lambda_i^0, \phi_j^0, \lambda_j^0) \\ \alpha_{ij}^0 &= \alpha(\phi_i^0, \lambda_i^0, \phi_j^0, \lambda_j^0) \end{aligned}$$



Observation equation of an azimuth

$$\alpha_{ij} - \varepsilon_{ij} = \alpha(\phi_i^0 + \delta\phi_i, \lambda_i^0 + \delta\lambda_i, \phi_j^0 + \delta\phi_j, \lambda_j^0 + \delta\lambda_j)$$

$$\approx \alpha_{ij}^0 + \frac{\partial \alpha}{\partial \phi_i} \cdot \delta\phi_i + \frac{\partial \alpha}{\partial \lambda_i} \cdot \delta\lambda_i + \frac{\partial \alpha}{\partial \phi_j} \cdot \delta\phi_j + \frac{\partial \alpha}{\partial \lambda_j} \cdot \delta\lambda_j$$

$$l_{ij} - \varepsilon_{ij} = c_i \cdot \delta\phi_i + d_i \cdot \delta\lambda_i + c_j \cdot \delta\phi_j + d_j \cdot \delta\lambda_j$$

$$\left. \begin{aligned} c_i &= M_i^0 \sin \alpha_{ij}^0 / s_{ij}^0 \\ d_i &= N_j^0 \cos \phi_j^0 \cos \alpha_{ji}^0 / s_{ij}^0 \\ c_j &= M_j^0 \sin \alpha_{ji}^0 / s_{ij}^0 \\ d_j &= -N_j^0 \cos \phi_j^0 \cos \alpha_{ji}^0 / s_{ij}^0 \end{aligned} \right\}$$

$$l_{ij} = \alpha_{ij} - \alpha_{ij}^0$$



Least squares adjustment on the ellipsoid

Final observation equations for all measurements

$$L - \varepsilon = A X$$

L : (reduced) measurements (azimuths, angles, distances etc)

ε : residuals (errors)

X : unknowns (coordinate corrections, other parameters)

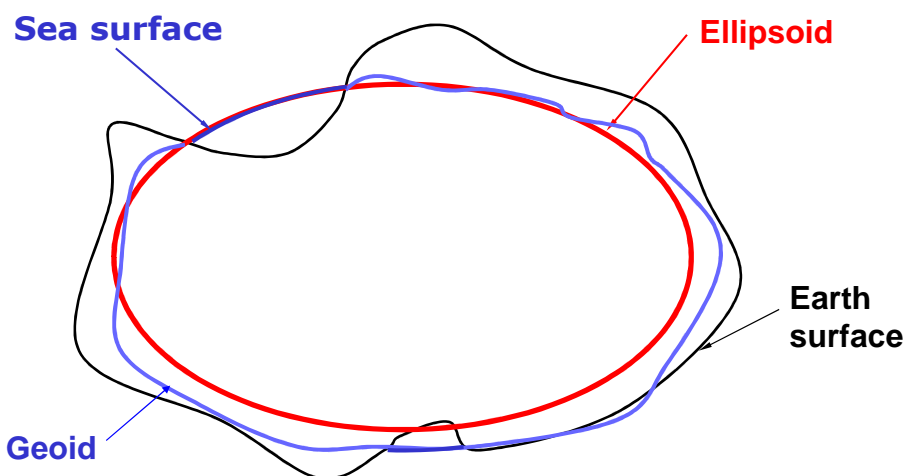
A : design matrix

Least squares solution:

$$\hat{X} = (A^T P A)^{-1} A^T P L$$

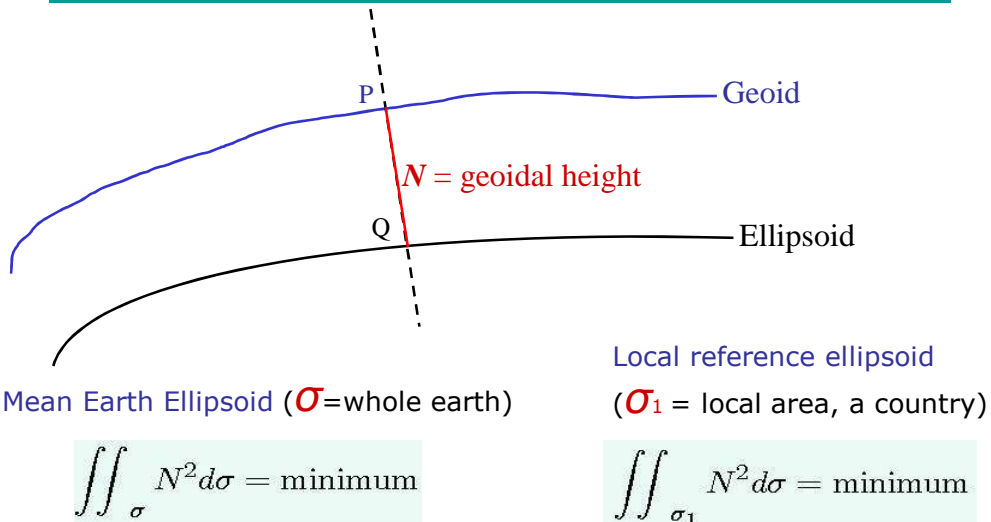


Fitting ellipsoids to the geoid

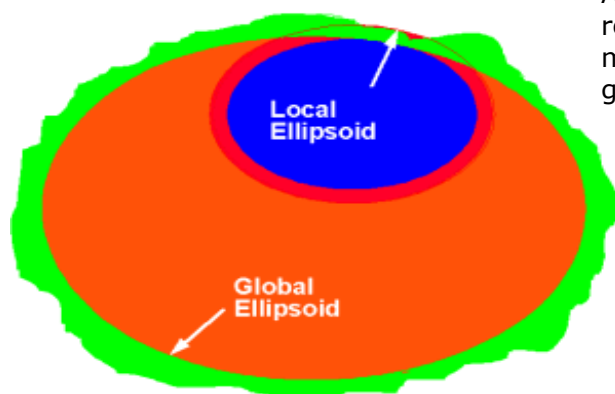




The geoid-ellipsoid separation



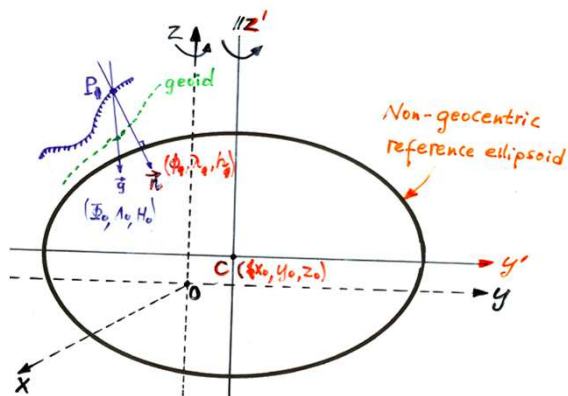
Local vs global ellipsoids



A locally fitted reference ellipsoid is most often **not** geocentric

A globally fitted reference ellipsoid (*Mean Earth Ellipsoid*) is geocentric

Positioning of the ellipsoid



O: geocentre
C: centre of the reference ellipsoid

Assumption: ellipsoid's minor axis Z' is parallel to the Earth rotation axis Z

→ the equator of the ellipsoid is parallel to the Earth's equator

→ position of the ellipsoid in relation to the earth can be defined by the geocentric coordinates (x_0, y_0, z_0) of the ellipsoidal center C

Geodetic datum (*ellipsoidal datum*)

- A *geodetic datum* consists of :
 - reference ellipsoid (size, shape and position defined) **and**
 - a set of ground triangulation points whose 2D geodetic coordinates (φ, λ) are computed with respect to this reference ellipsoid
- **Assumptions:**
 - The minor axis of ellipsoid is parallel to earth rotation axis;
 - The initial median plane of the ellipsoid is parallel to the *astronomical* Greenwich meridian plane
- The position of the ellipsoid is often/best defined by the geocentric coordinates of the ellipsoidal centre (x_0, y_0, z_0)

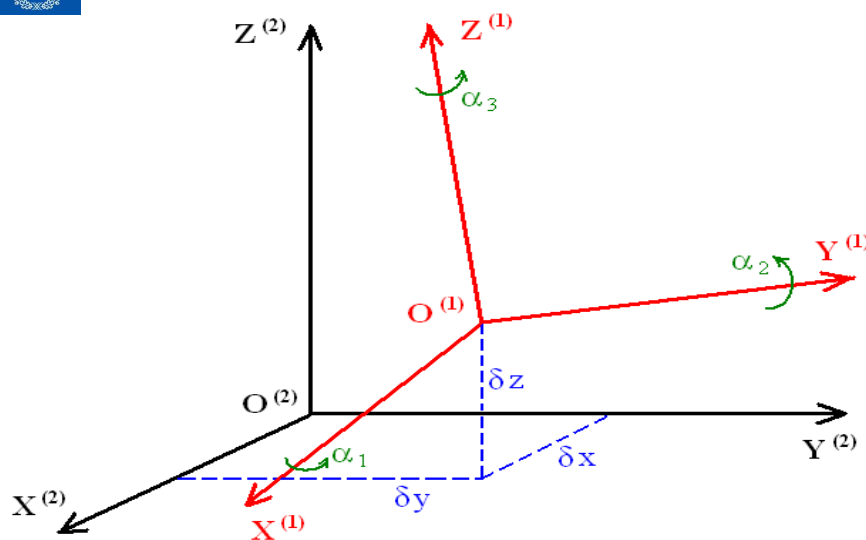


Problems with astrogeodetic systems

- National/regional astrogeodetic systems are often non-geocentric
- Different ellipsoids and map projections in use
- Accuracy of triangulation coordinates are lower than modern GNSS
- Transformation between triangulation- and GNSS-based coordinate systems is needed during transition period
- Methods, procedures and services for coordinate transformation should be developed/provided.



Two 3D coordinate systems





Helmert model of 3D transformation

$$\begin{bmatrix} X_i^{(2)} \\ Y_i^{(2)} \\ Z_i^{(2)} \end{bmatrix} = \begin{bmatrix} \delta x \\ \delta y \\ \delta z \end{bmatrix} + s \cdot R(\alpha_1, \alpha_2, \alpha_3) \cdot \begin{bmatrix} X_i^{(1)} \\ Y_i^{(1)} \\ Z_i^{(1)} \end{bmatrix}$$

$$s = 1 + \delta s$$

$$R_{3 \times 3} = R(\alpha_1, \alpha_2, \alpha_3) = R_3(\alpha_3) \cdot R_2(\alpha_2) \cdot R_1(\alpha_1)$$

$$= \begin{bmatrix} \cos \alpha_3 & \sin \alpha_3 & 0 \\ -\sin \alpha_3 & \cos \alpha_3 & 0 \\ 0 & 0 & 1 \end{bmatrix} \cdot \begin{bmatrix} \cos \alpha_2 & 0 & -\sin \alpha_2 \\ 0 & 1 & 0 \\ \sin \alpha_2 & 0 & \cos \alpha_2 \end{bmatrix} \cdot \begin{bmatrix} 1 & 0 & 0 \\ 0 & \cos \alpha_1 & \sin \alpha_1 \\ 0 & -\sin \alpha_1 & \cos \alpha_1 \end{bmatrix}$$

$$= \begin{bmatrix} \cos \alpha_2 \cos \alpha_3 & \cos \alpha_1 \sin \alpha_3 + \sin \alpha_1 \sin \alpha_2 \cos \alpha_3 & \sin \alpha_1 \sin \alpha_3 - \cos \alpha_1 \sin \alpha_2 \cos \alpha_3 \\ -\cos \alpha_2 \sin \alpha_3 & \cos \alpha_1 \cos \alpha_3 - \sin \alpha_1 \sin \alpha_2 \sin \alpha_3 & \sin \alpha_1 \cos \alpha_3 + \cos \alpha_1 \sin \alpha_2 \sin \alpha_3 \\ \sin \alpha_2 & -\sin \alpha_1 \cos \alpha_2 & \cos \alpha_1 \cos \alpha_2 \end{bmatrix}$$

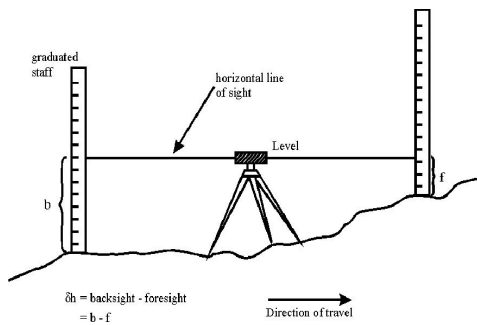


GRS overview

- Why? What is new?
- Astrogeodetic triangulation (2D)
- Height systems from levelling (1D)
- 3D geocentric reference systems
- Geodetic infrastructure in Sweden



Height differences from levelling

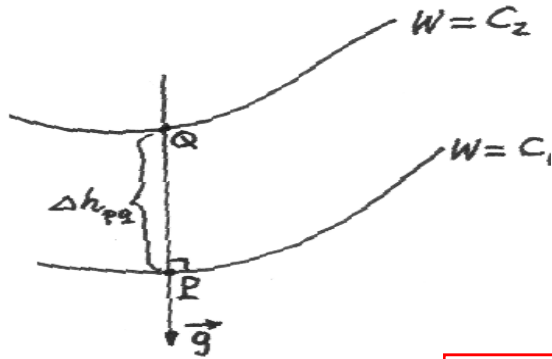


Swedish Motorized Levelling





Distance between two close equipotential surfaces



$$g = |\vec{g}| = -\frac{dW}{dn}$$

$$g_p \approx -\Delta W_{pq} / \Delta h_{pq}$$

$$\Delta h_{pq} \approx -\Delta W_{pq} / g_p$$

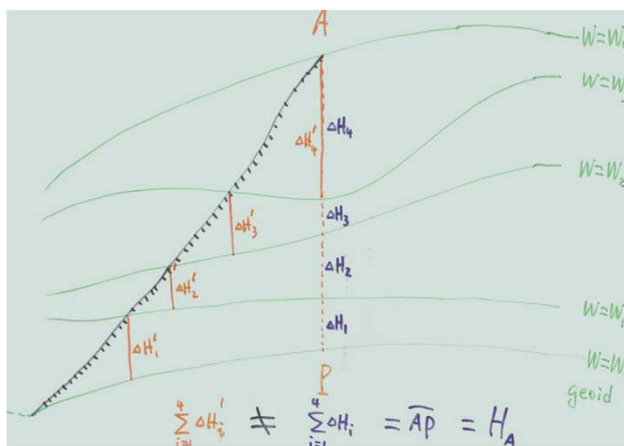
$$\begin{aligned} \Delta W_{pq} &= W_q - W_p \\ &= C_2 - C_1 = \text{constant} \end{aligned}$$

→ Equipotential surfaces are not parallel !!!



Theoretical misclosure (ϵ) of levelling

→ Equipotential surfaces are not parallel !!!



Levelling between two benchmarks along 2 different routes show different height differences

Levelling along a closed loop is not equal to zero:

$$\oint dh = \epsilon \neq 0;$$

→ ϵ : theoretical misclosure



Height Systems

Geopotential number of point A:

$$C_A = W_0 - W_A$$

Orthometric height of point A:

$$H_A = \frac{C_A}{\bar{g}_A} = \frac{W_0 - W_A}{\bar{g}_A}$$

Normal height of point A:

$$H_A^* = \frac{C_A}{\bar{\gamma}_A} = \frac{W_0 - W_A}{\bar{\gamma}_A}$$

Dynamic height of point A:

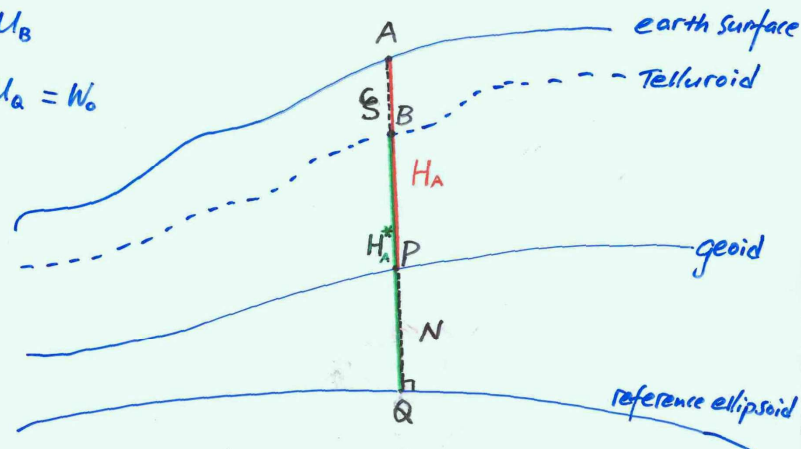
$$H_A^d = \frac{C_A}{\gamma_0} = \frac{W_0 - W_A}{\gamma_0}$$



Concepts of Height Systems

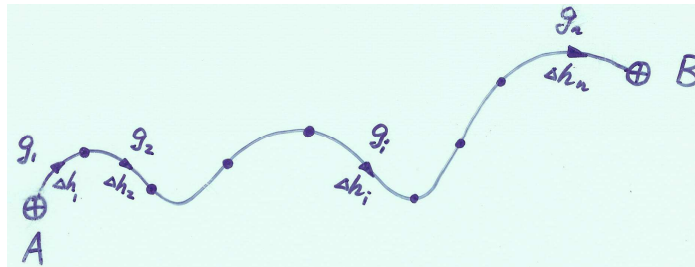
$$W_A = U_B$$

$$W_P = U_A = W_0$$



$$h = \xi + H^* = N + H$$

Principle of Precise levelling



Given: H_A^* , g_i , Δh_i ($i=1, 2, 3, \dots, n$)
Sought: $H_B^* = ?$

Principle of Precise levelling



Solution:

1. $C_A = H_A^* \cdot \bar{\tau}_A$
2. $\Delta C_{AB} \triangleq C_B - C_A = \sum_{i=1}^n (g_i \cdot \Delta h_i)$
3. $C_B = C_A + \Delta C_{AB}$
4. $H_B^* = \frac{C_B}{\bar{\tau}_B}$

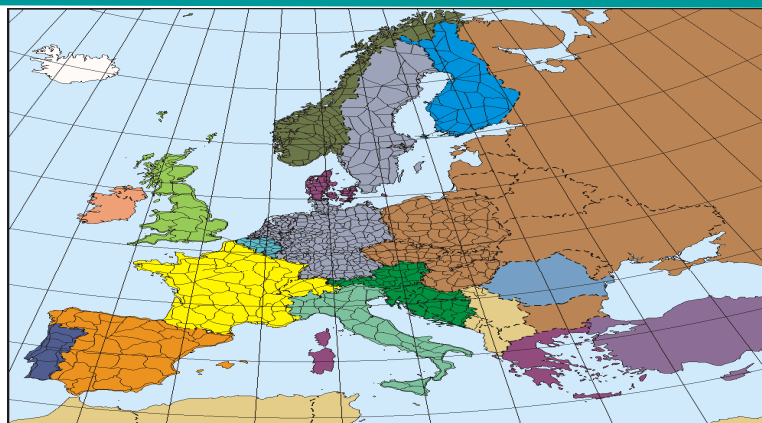


Problems with height systems

- Orthometric heights are not possible to determine. Only geopotential differences can be uniquely determined.
Gravity data is needed !
- Different height systems in use
- Different zero datums
- Local height systems are not connected.



Zero height datums in Europe

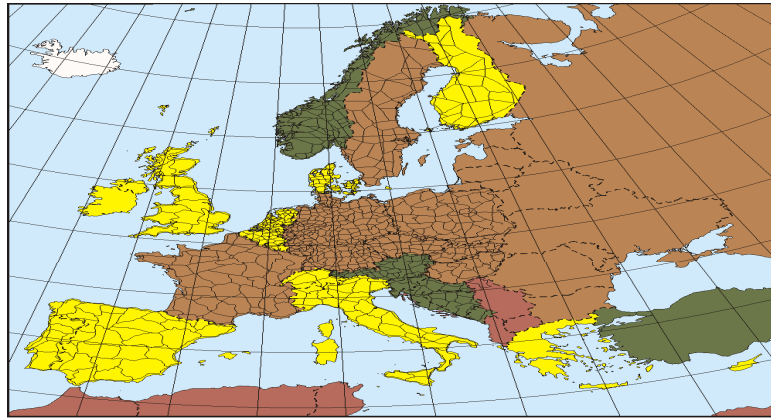


~ UELN lines

Alcant	Constanta	Malin Head	Tregde
Amsterdam	Genova	Marseille	Trieste
Antalya	Helsinki	Newlyn	no information
Cascais	Kronstadt	Ostend	other



Height systems in Europe

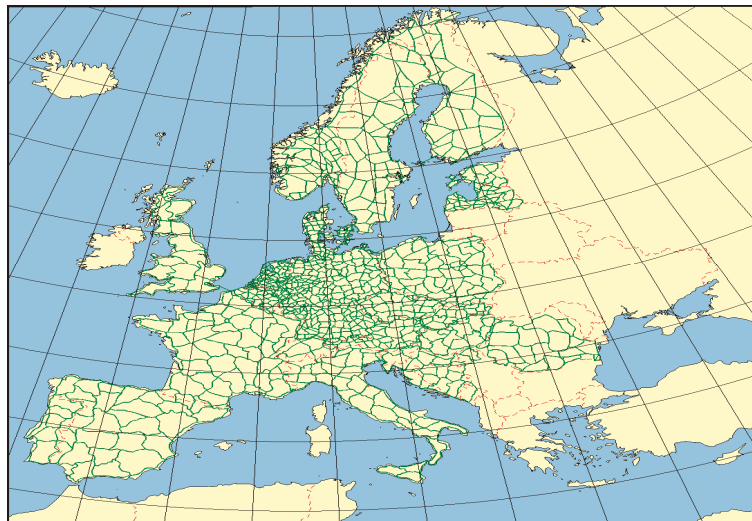


Kind of Heights

- normal heights
- orthometric heights
- normal orthometric heights
- no information
- no levelling heights
- ~ UELN lines

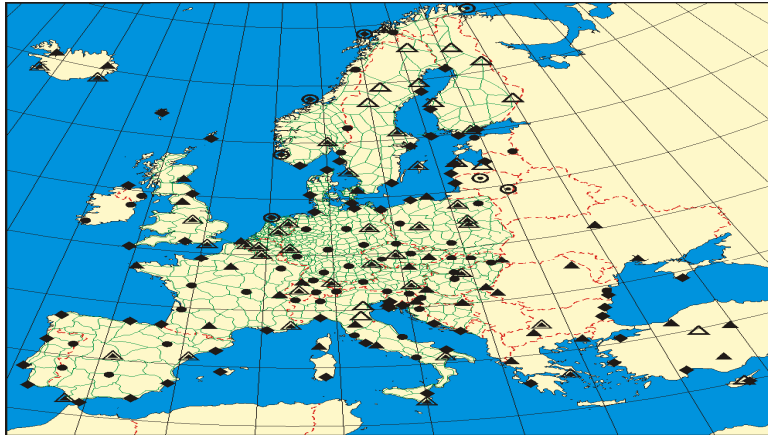


United European Levelling Network (UELN95/98)





EUVN – EUropean Vertical GPS Network



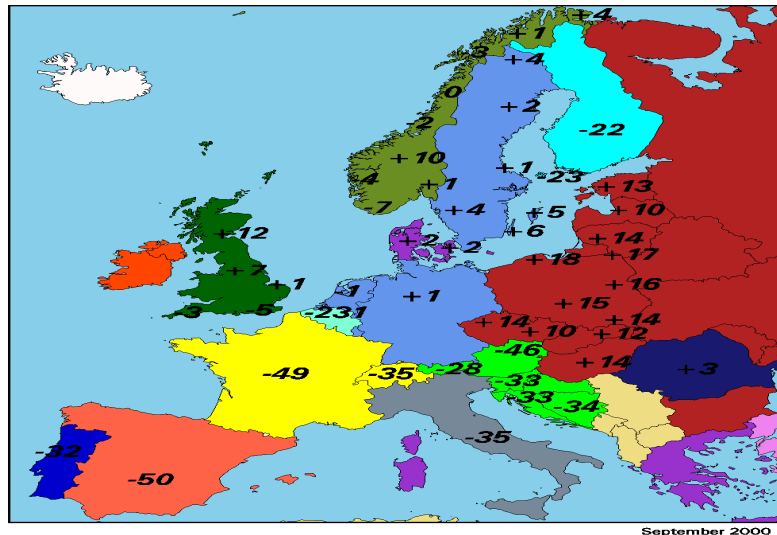
- ▲ EUREF sites
- ▲ GPS permanent stations - EUREF
- ▲ GPS permanent stations
- UELN & UPLN nodal points
- ⊙ GPS permanent stations - nodal points
- ◆ Tide gauge sites
- ⊙ GPS permanent stations - tide gauge
- ∨ UELN lines



European Vertical Reference System 2000

- United European Levelling Network
 - Only levelling lines between nodal points are included
 - Different realizations of the mean sea level
 - Different height systems
- EUropean Vertical GPS Network
 - Connect different height system in one common system using GPS
 - Including: 66 EUREF GPS stations, 54 UELN nodal points and 63 Mareograph/tide gauge stations
- **European Vertical Reference System 2000 (EVRS 2000)**
 - Zero datum at Amsterdam: $W_0 = (W_0)_{NAP} = (U_0)_{GRS80}$
 - Heights expressed as geopotential numbers. Normal heights used
 - Zero tidal system

Height Differences: National vs EVRS 2000



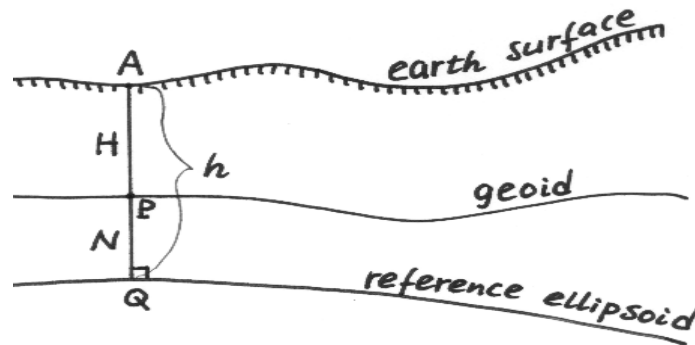
Problems with height systems

- Orthometric heights are not possible to determine. Only geopotential can be uniquely determined.
- Different height systems in use
- Different zero datums
- Local height systems are not connected.
- GNSS-derived ellipsoid heights are not related to the geoid (MSL).



Ellipsoidal heights vs heights above MSL

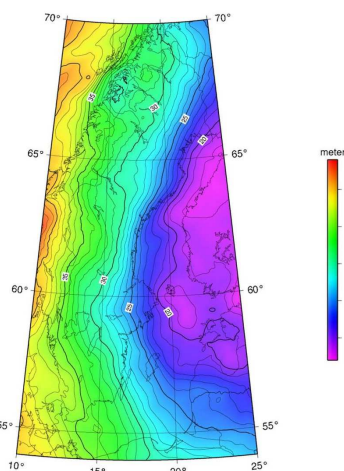
$$h=H+N$$



- GPS survey provides ellipsoidal heights (h)
- Mapping and construction requires heights above the geoid (H)
- To know the geoid heights (N), one has to know the gravity field



Gravimetric Geoid Model - SWEN 08LR



- Computed using the KTH method by modifying Stokes' formula and combining global gravitational models with terrestrial gravity data.
- Fitted to 1570 GPS/levelling points (SWEREF 99 / RH 2000)
- Remaining residuals are smoothly interpolated
- A height transformation model from SWEREF 99 to RH 2000
- Accuracy:
 - 1 - 1.5 cm in most areas
 - 5 - 10 cm in Northwest without levelling

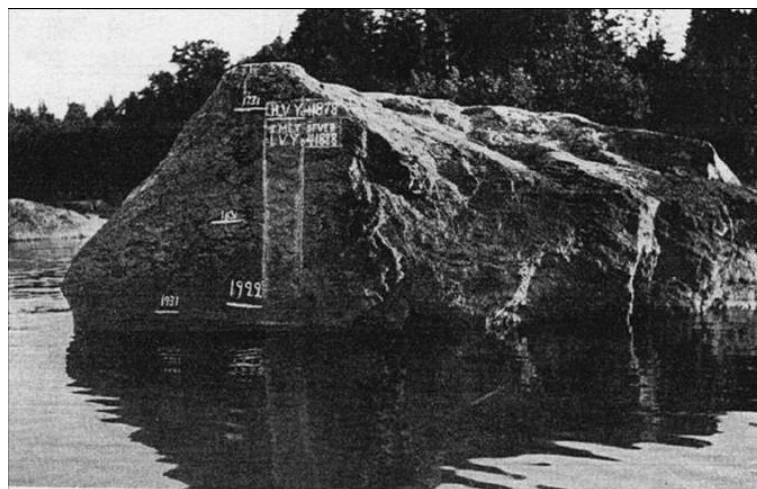


Problems with height systems

- Orthometric heights are not possible to determine. Only geopotential can be uniquely determined.
- Different height systems in use
- Different zero datums
- Local height systems are not connected.
- GNSS-derived ellipsoidal heights are not related to the geoid (MSL).
- Vertical crustal movement affects heights



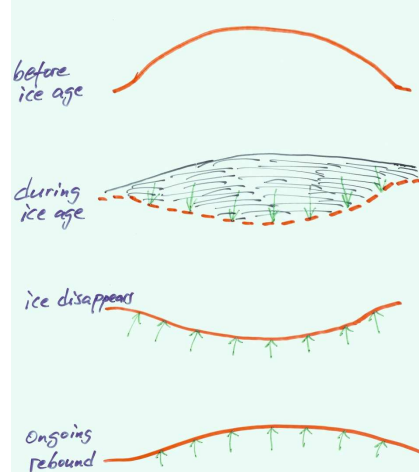
Land uplift or water sinking ???



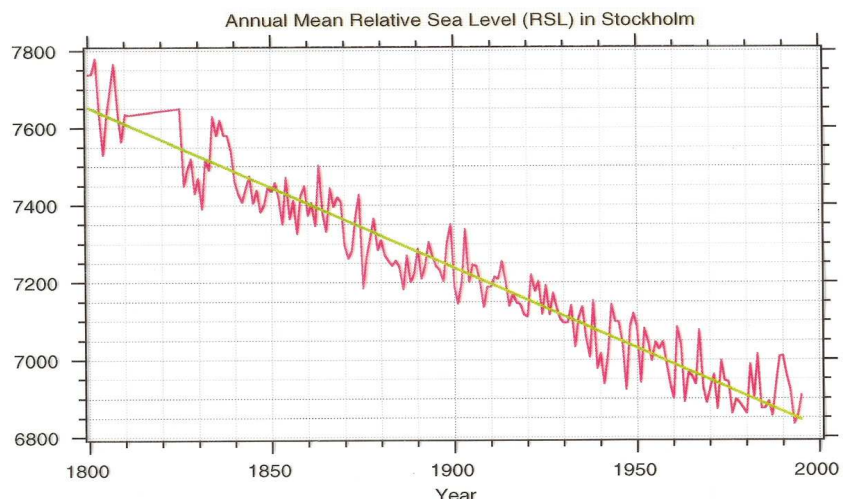
Anders Celsius 1765



Land uplift - postglacial rebound

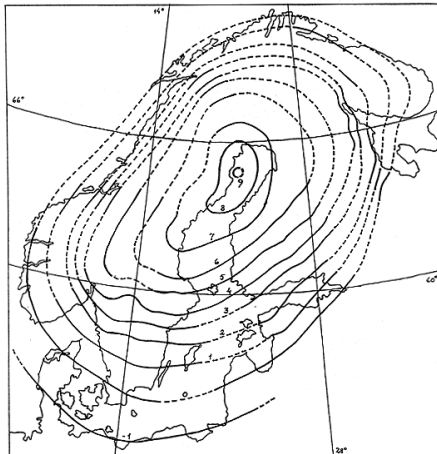


Relative sea level at tide gauge

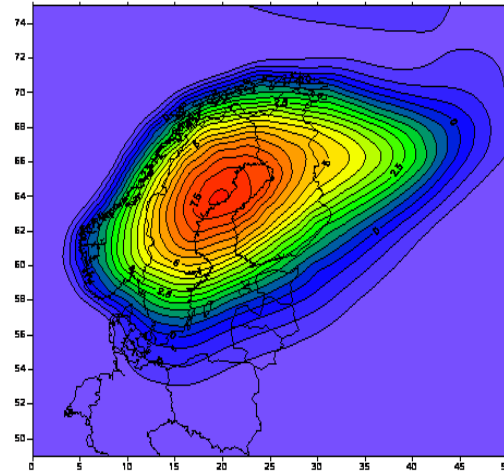




Swedish land uplift model (mm/year)



Martin Ekman (1998)



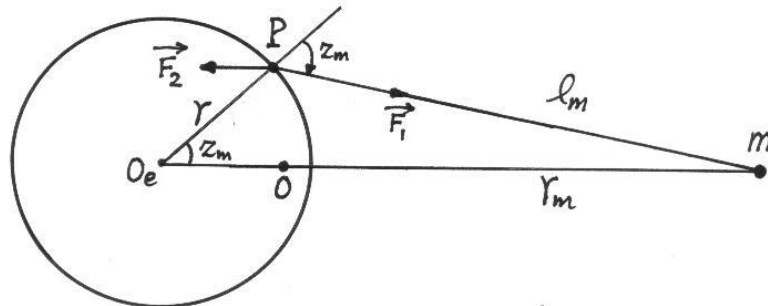
RH 2000 LU



Problems with height systems

- Orthometric heights are not possible to determine. Only geopotential can be uniquely determined.
- Different height systems in use
- Different zero datums
- Local height systems are not connected.
- GNSS-derived ellipsoidal heights are not related to the geoid (MSL).
- Vertical crustal movement affects heights
- Treatment of permanent tide in height definition

Tidal potential of the Moon



$$T^m = \frac{Gm}{l_m} - \left(\frac{Gm}{r_m} + Gm \frac{r}{r_m^2} \cos z_m \right)$$

Non-zero time-average

$$T_2 = D \left[\left(\frac{1}{3} - \sin^2 \phi \right) + 3 \left(\sin^2 \phi - \frac{1}{3} \right) \sin^2 \delta + \cos^2 \phi \cos^2 \delta \cos 2t + \sin 2\phi \sin 2\delta \cos t \right]$$

$$T_2 = D (T_z + T_s + T_t)$$

$$\overline{T_s} = \frac{1}{2\pi} \int_0^{2\pi} D \cdot T_s dt = 0$$

$$\overline{T_t} = \frac{1}{2\pi} \int_0^{2\pi} D \cdot T_t dt = 0$$

$$\overline{T_z} = \frac{1}{2\pi} \int_0^{2\pi} D \cdot T_z dt = D \left(\frac{1}{2} \sin^2 \epsilon - \frac{1}{3} \right) (\sin^2 \phi - \frac{1}{3})$$

The time-average of the tidal potential T_2 does not vanish. This causes permanent tidal effects, including permanent deformation of the solid earth.

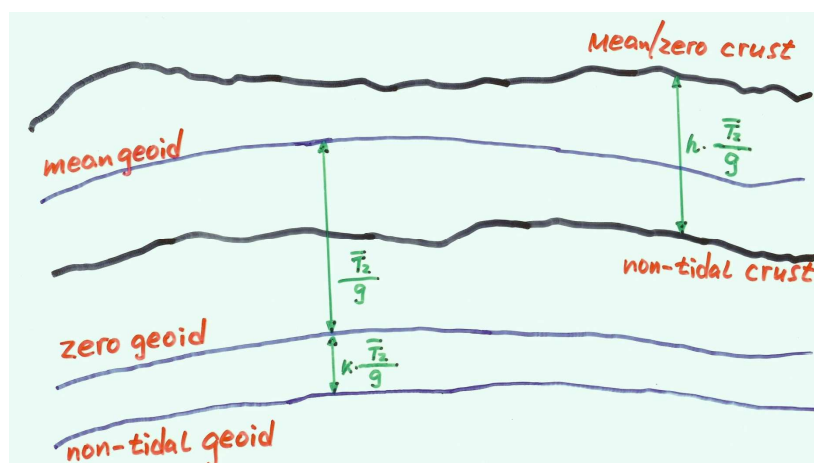


Permanent tide and tidal systems

- 4 parts of tidal effects
 - Direct (extra-terrestrial masses) vs indirect effects (earth's deformation)
 - Periodic vs permanent effects. Periodic effects can be averaged out.
 - Treatment of the permanent tide leads to 3 tidal systems
- Non-tidal model (Tide-free) (GNSS - ITRF)
 - Permanent deformation of the earth is *eliminated* → *earth's form changed*
 - Permanent tidal effects are *eliminated* from the measurements
- Zero tidal model (EVRS 2000)
 - Permanent deformation of the earth is *retained* → *masses outside the geoid*
 - Permanent tidal effects are *eliminated* in the measurements
- Mean tidal model
 - Permanent deformation of the earth is *retained*
 - Permanent tidal effects are *retained* → *included in the normal gravity field ?*



Tidal Systems: Non-Tidal, Mean vs Zero Tide





Tidal Systems: geoidal differences

Latitude	$3 \sin(\text{FI})^2 - 1$	Geoid separation, cm	Geoid separation, cm	Crust separation, cm				
(degree)		Non-tidal - Zero	Zero - Mean	Non-tidal - Mean/zero				
0	-1,000000000	6,80	2,04	4,21	PI/180	0,017453293	D = 26206	cm**2/s**2
5	-0,977211630	6,64	1,99	4,12	YPSILON, deg	23,439291	g= 980	cm/s**2
10	-0,909538931	6,18	1,85	3,83	sin (YPSILON)	0,3977772	D/g 26,741	cm
15	-0,799038106	5,43	1,63	3,37			TO -6,79805	
20	-0,649066665	4,41	1,32	2,74				
25	-0,464181415	3,16	0,95	1,96				
30	-0,250000000	1,70	0,51	1,05				
35	-0,013030215	0,09	0,03	0,05				
40	0,239527733	-1,63	-0,49	-1,01				
45	0,500000000	-3,40	-1,02	-2,11				
50	0,760472267	-5,17	-1,55	-3,21				
55	1,013030215	-6,89	-2,07	-4,27				
60	1,250000000	-8,50	-2,55	-5,27				
65	1,464181415	-9,95	-2,99	-6,17				
70	1,649066665	-11,21	-3,36	-6,95				
75	1,799038106	-12,23	-3,67	-7,58				
80	1,909538931	-12,98	-3,89	-8,05				
85	1,977211630	-13,44	-4,03	-8,33				
90	2,000000000	-13,60	-4,08	-8,43				



GRS overview

- Why? What is new?
- Astrogeodetic triangulation (2D)
- Height systems from levelling (1D)
- 3D geocentric reference systems
- Geodetic infrastructure in Sweden



Celestial vs terrestrial reference system

- **Celestial reference system:** a reference system which is fixed in space with respect to stars and other planets
- **Terrestrial reference system:** a reference system which is fixed with respect to the earth's body

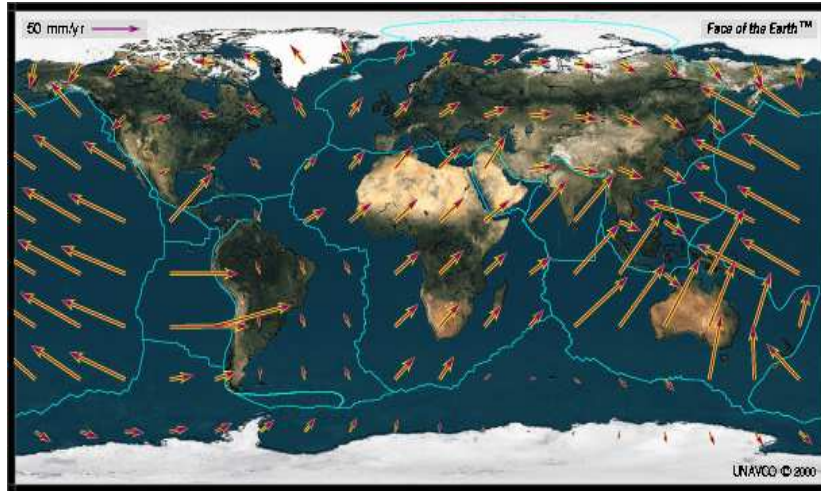


Astro-geodynamic effects

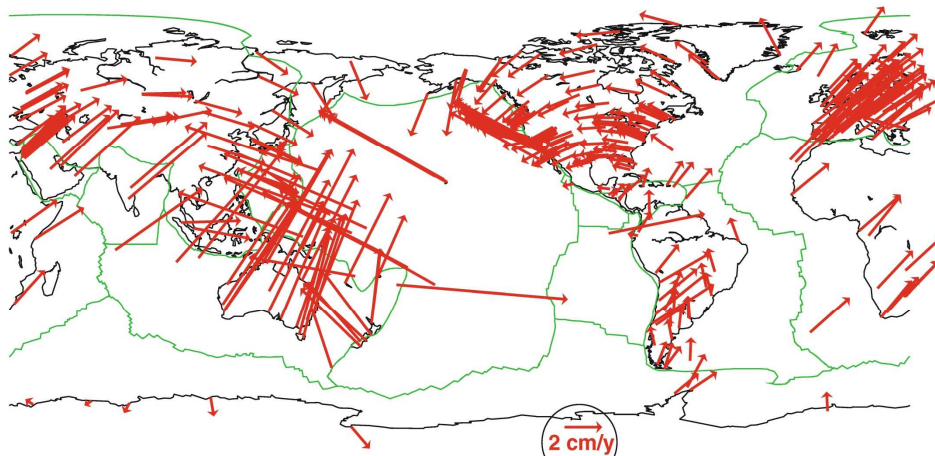
- Global tectonic movement (*continental drift*)
 - time-dependent coordinates: TRF $xxxx$.
 - coordinates + velocities: $x, y, z, \dot{x}, \dot{y}, \dot{z}$
- Changes in earth rotation
 - Polar motion → affecting TRS
 - Precession and nutation → affecting CRS
 - Length of Day (LoD) → affecting CRS and TRS



Tectonic plate model NNR NUVEL-1A



Horizontal velocity from ITRF 2008



406 stations have formal errors < 0.2mm/year

Polar motion

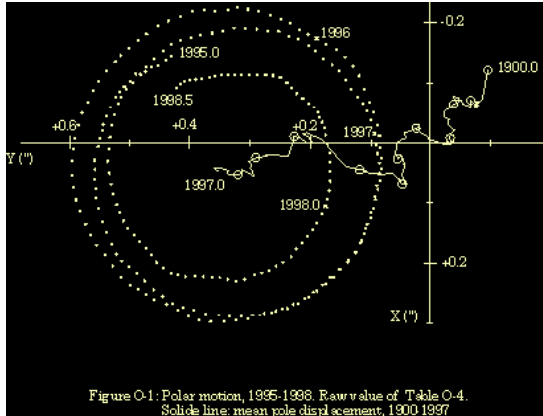
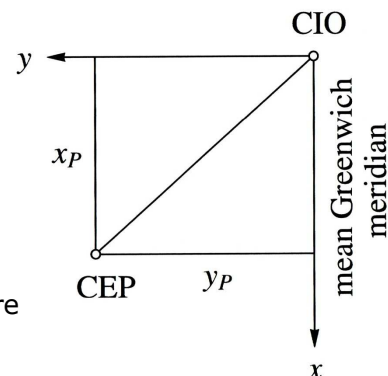


Figure O-1: Polar motion, 1995-1998. Raw value of Table O-4.
Solid line: mean pole displacement, 1900-1997

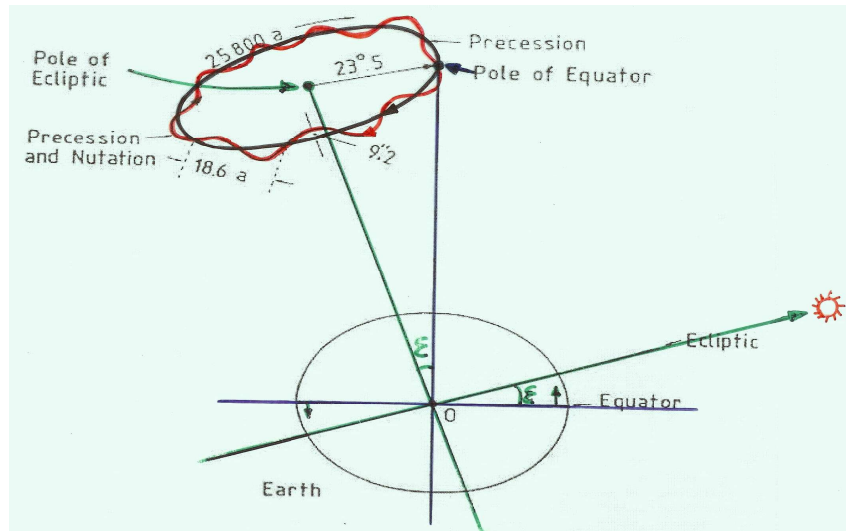
- Euler period: 305 days
- Chandler period: 430d
- Approx. radius: 8m
- Daily radius: 0,5m

Conventional International Origin (CIO)

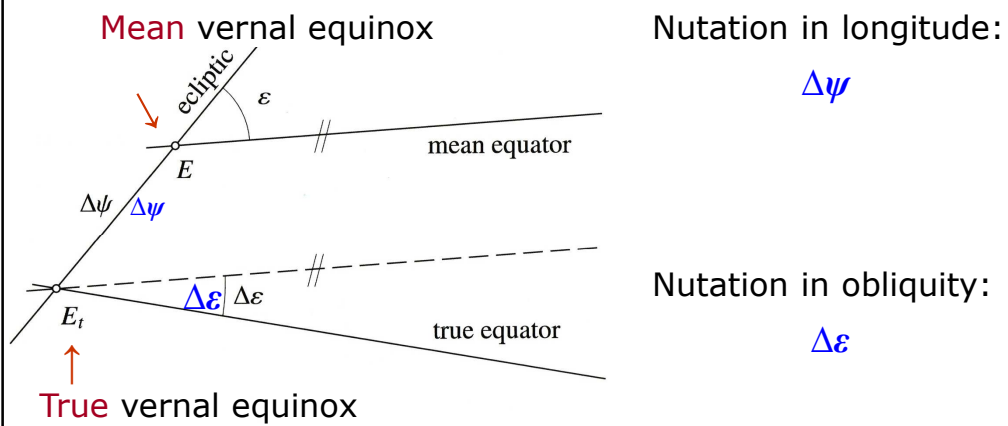
- CIO = mean position of the North Pole 1900-1905
- The instantaneous celestial ephemeris pole (CEP) is defined by polar coordinates (x_p, y_p)
- Convention: measurements w.r.t instantaneous CEP, while coordinates are defined w.r.t. CIO



Precession and nutation

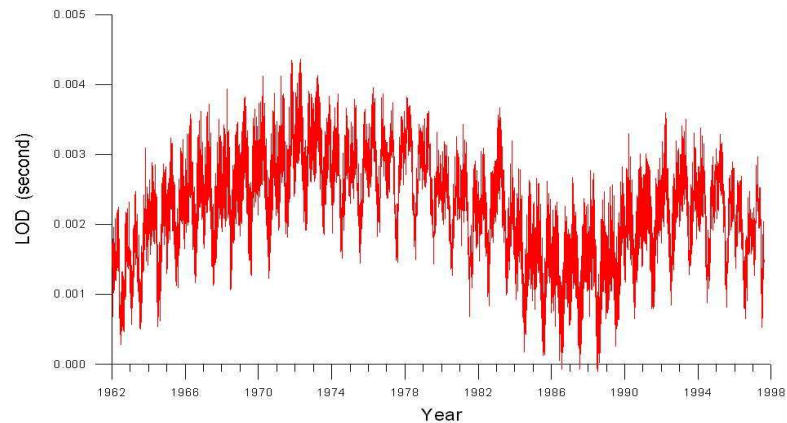


Nutation parameters





Variation in Length of Day (LoD)

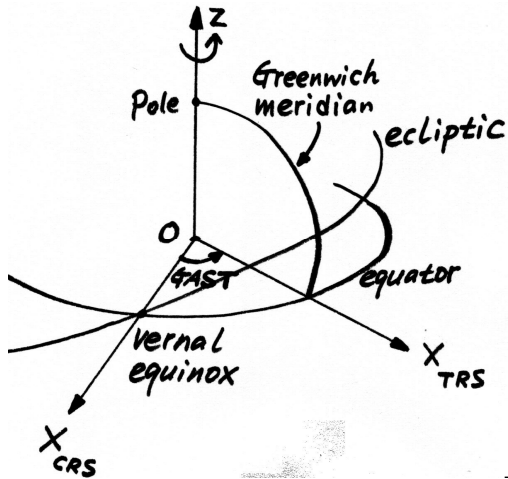


Earth Orientation Parameters (EOP)

- 2 Polar coordinates with respect to **CIO** : x_p, y_p
- 2 Nutation parameters :
 - nutation in obliquity ($\Delta\varepsilon$)
 - nutation in longitude ($\Delta\psi$)
- 1 time parameter
 - **UTC-UT1** (provide UT1 when UTC is known)



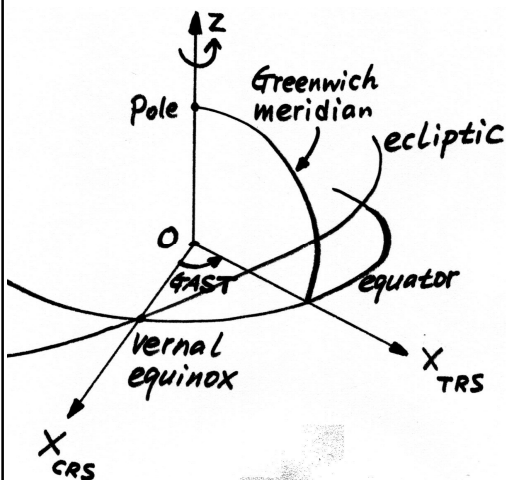
International Celestial Reference System (ICRS)



- Origin at **barycentre** (practically at geocentre)
- Z-axis toward the **mean celestial North Pole at J2000.0**
- X-axis toward the **mean equinox at J2000.0**
- Y-axis follows to form a right-handed system

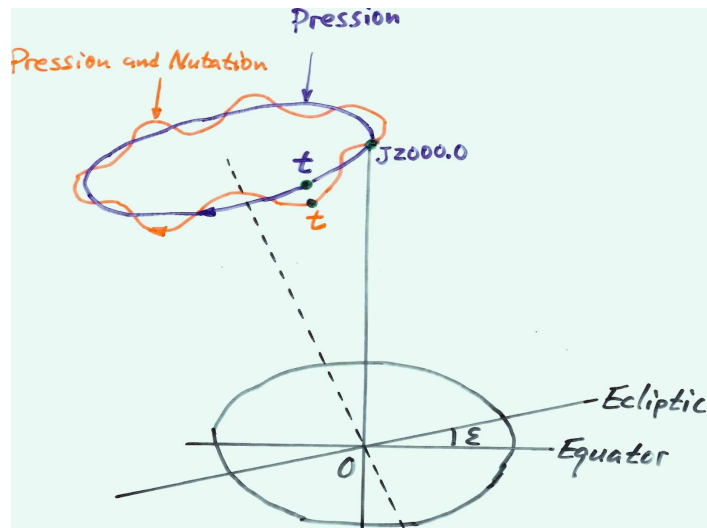


International Terrestrial Reference System (ITRS)



- Origin at the earth's centre of masses including oceans and atmosphere
- Z-axis toward the **CIO**
- X-axis along intersection of the equator and the **Greenwich Meridian**
- Y-axis inside the equator to form a right-handed system

Reduction of precession and nutation



Transformation from ICRS to ITRS

$$\begin{bmatrix} x \\ y \\ z \end{bmatrix}_{ITRS} = R_p(x_p, y_p) \cdot R_3(GAST) \cdot N(\varepsilon_0, \Delta\varepsilon, \Delta\psi) \cdot P(z, \vartheta, \xi) \cdot \begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ICRS}$$

$$P(z, \vartheta, \xi) = R_3(-z) \cdot R_2(\vartheta) \cdot R_3(-\xi)$$

$$= \begin{bmatrix} +\cos z \cos \vartheta \cos \xi - \sin z \sin \xi & -\cos z \cos \vartheta \sin \xi - \sin z \cos \xi & -\cos z \sin \vartheta \\ +\sin z \cos \vartheta \cos \xi + \cos z \sin \xi & -\sin z \cos \vartheta \sin \xi + \cos z \cos \xi & -\sin z \sin \vartheta \\ +\sin \vartheta \cos \xi & -\sin \vartheta \sin \xi & +\cos \vartheta \end{bmatrix}$$

$$N_{3 \times 3}(\varepsilon_0, \Delta\varepsilon, \Delta\psi) = R_1(-\varepsilon) \cdot R_3(-\Delta\psi) \cdot R_1(\varepsilon_0) \quad (\varepsilon = \varepsilon_0 + \Delta\varepsilon)$$

$$= \begin{bmatrix} \cos \Delta\psi & -\sin \Delta\psi \cos \varepsilon_0 & -\sin \Delta\psi \sin \varepsilon_0 \\ \sin \Delta\psi \cos \varepsilon & +\cos \Delta\psi \cos \varepsilon \cos \varepsilon_0 + \sin \varepsilon \sin \varepsilon_0 & +\cos \Delta\psi \cos \varepsilon \sin \varepsilon_0 - \sin \varepsilon \cos \varepsilon_0 \\ \sin \Delta\psi \sin \varepsilon & +\cos \Delta\psi \sin \varepsilon \cos \varepsilon_0 - \cos \varepsilon \sin \varepsilon_0 & +\cos \Delta\psi \sin \varepsilon \sin \varepsilon_0 + \cos \varepsilon \cos \varepsilon_0 \end{bmatrix}$$



Computation of *GAST*

$$\varepsilon_0 = 23^{\circ} 26' 21.448'' - 46.8150'' \cdot T - 0.00059'' \cdot T^2 + 0.001813'' \cdot T^3$$

$$\left. \begin{aligned} \xi &= 2306.2181 \cdot T + 0.30188 \cdot T^2 + 0.017998 \cdot T^3 \\ z &= 2306.2181 \cdot T + 1.09468 \cdot T^2 + 0.018203 \cdot T^3 \\ \vartheta &= 2004.3109 \cdot T - 0.42665 \cdot T^2 - 0.041833 \cdot T^3 \end{aligned} \right\} \text{ (unit: arcsecond)}$$

$$T = \frac{JD(t) - J2000.0}{36525} = \frac{JD(t) - 2\,451\,545.0}{36525}$$

$$GAST = (GMST)_0 + 1.002\,737\,909\,35 \cdot UT1 + \Delta\psi \cos \varepsilon_0$$
$$(GMST)_0 = 24\,110.54841^s + 8\,640\,184.812866^s T_0 + 0.093\,104^s T_0^2 - 6.2^s \cdot 10^{-6} T_0^3$$

$$T_0 = \frac{JD_{UT=0} - 2\,451\,545.0}{36525}$$



IERS

- IERS, **International Earth Rotation Service**, was established in 1987 by IAU and IUGG to replace
 - International Polar Motion Service (IPMS)
 - Bureau International de l'Heure (BIH)
- Renamed in 2003 as **International Earth Rotation and Reference Systems Service**



International Celestial Reference Frame (ICRF)

- First realization of ICRS, **ICRF1**, is made by IAU in 1997 based on VLBI measurements of 212 defining radio sources
- Two extensions of ICRF1 are made by IERS in 1999 and 2004
- Second realization of ICRS, **ICRF2**, is made in 2009
 - VLBI measurements of 3414 radio sources
 - 295 defining radio sources



International Celestial Reference Frame (**ICRF2**)

Coordinates of 295 ICRF2 defining sources

ICRF Designation (1)	IERS Des. (2)	Right Ascension			Declination			Uncertainty	
		J2000.0			J2000.0			R.A.	Dec.
		h	m	s	o	'	''	s	''
ICRF J000435.6-473619	0002-478	00	04	35.65550384	-47	36	19.6037899	0.00001359	0.0002139
ICRF J001031.0+105829	0007+106	00	10	31.00590186	10	58	29.5043827	0.00000491	0.0000930
ICRF J001101.2-261233	0008-264	00	11	01.24673846	-26	12	33.3770171	0.00000660	0.0000936
ICRF J001331.1+405137	0010+405	00	13	31.13020334	40	51	37.1441040	0.00000482	0.0000683
ICRF J001611.0-001512	0013-005	00	16	11.08855479	-00	15	12.4453413	0.00000435	0.0001005
ICRF J001945.7+732730	0016+731	00	19	45.78641940	73	27	30.0174396	0.00000989	0.0000424
ICRF J002232.4+060804	0019+058	00	22	32.44120914	06	08	04.2690807	0.00000439	0.0000956
ICRF J003824.8+413706	0035+413	00	38	24.84359231	41	37	06.0003032	0.00000499	0.0000613
ICRF J005041.3-092905	0048-097	00	50	41.31738756	-09	29	05.2102688	0.00000278	0.0000428
ICRF J005109.5-422633	0048-427	00	51	09.50182012	-42	26	33.2932480	0.00000932	0.0001177
ICRF J010245.7+582411	0059+581	01	02	45.76238248	58	24	11.1366009	0.00000523	0.0000414
ICRF J010645.1-403419	0104-408	01	06	45.10796851	-40	34	19.9602291	0.00000376	0.0000455
ICRF J010915.4-604948	0107-610	01	09	15.47520598	-60	49	48.4599686	0.00001744	0.0001750
ICRF J011205.8+224438	0109+224	01	12	05.82471754	22	44	38.7863909	0.00000379	0.0000653
ICRF J011327.0+494824	0110+495	01	13	27.00680344	49	48	24.0431742	0.00000597	0.0000727
ICRF J011857.2-214130	0116-219	01	18	57.26216666	-21	41	30.1399986	0.00000683	0.0001138
ICRF J012141.5+114950	0119+115	01	21	41.59504339	11	49	50.4131012	0.00000279	0.0000429
ICRF J013305.7-520003	0131-522	01	33	05.76255607	-52	00	03.9457209	0.00001218	0.0001605



International Terrestrial Reference Frame

- ITRF is realization of ITRS
- ITRF networks consists of stations all over the world, measured by 4 space geodetic methods
 - 1) Very Long Baseline Interferometry (VLBI)
 - 2) Satellite Laser Ranging (SLR)
 - 3) GNSS
 - 4) Doppler Orbitography Integrated by Satellites (DORIS)
- Final products include:
 - Position **coordinates** and **velocities** for a reference epoch
 - **Transformation parameters** and their time derivatives
 - Consistent estimates of 5 **EOP**

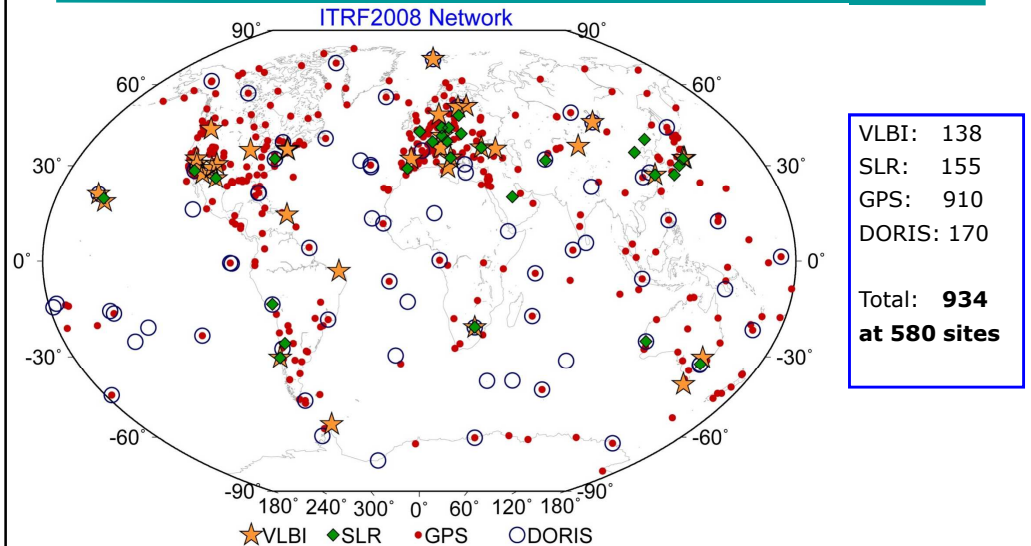


List of ITRF solutions

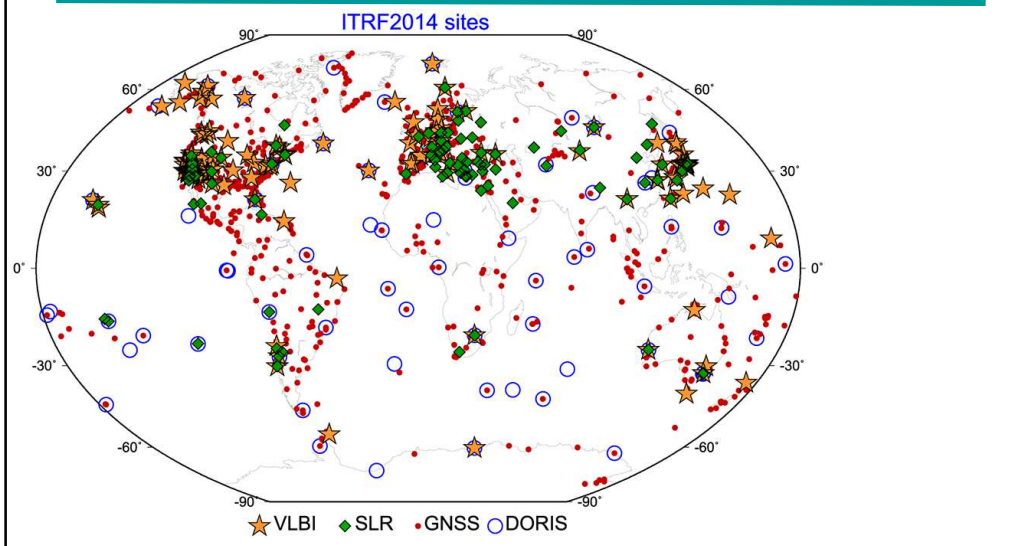
- | | |
|-------------------|--------------------------|
| • ITRF88 | • 1988.0 reference epoch |
| • ITRF89 | • 1988.0 |
| • ITRF92 | • 1988.0 |
| • ITRF93 | • 1988.0 |
| • ITRF94 | • 1997.0 |
| • ITRF96 | • 1997.0 |
| • ITRF97 | • 1997.0 |
| • ITRF2000 | • 1997.0 |
| • ITRF2005 | • 2000.0 |
| • ITRF2008 | • 2005.0 |
| • ITRF2014 | • 2010.0 |



Global Network of ITRF 2008



Global Network of ITRF 2014





ITRF 2008

- Computations in 3 steps
 - 1) Each type (VLBI,SLR,GPS,DORIS) is processed separately to produce weekly (daily VLBI) time series of positions and EOP
 - 2) Above results are combined to estimate position coordinates for epoch 2005.0, velocity and daily EOP
 - 3) Combination with local survey at co-locating stations
- ITRF 2008 results:
 - Origin and scale are defined by SLR data
 - Orientation is defined by average of VLBI and SLR data
 - Scale accuracy: **1 ppb, 0.05 ppb/year** (VLBI-SLR agreement)
 - Origin accuracy: at the level of or better than **1 cm**



Station coordinates/velocities of ITRF 2008

10	VELX	7205	A	1	05:001:00000	m/y	2	-0.154701843128797E-01	0.47543E-04
11	VELY	7205	A	1	05:001:00000	m/y	2	-0.121094247184766E-02	0.62981E-04
12	VELZ	7205	A	1	05:001:00000	m/y	2	0.408529863128065E-02	0.69402E-04
13	STAX	7204	A	1	05:001:00000	m	2	0.882879772507557E+06	0.13845E-02
14	STAY	7204	A	1	05:001:00000	m	2	-0.492448231271830E+07	0.35634E-02
15	STAZ	7204	A	1	05:001:00000	m	2	0.394413070534473E+07	0.27791E-02
16	VELX	7204	A	1	05:001:00000	m/y	2	-0.143669405946909E-01	0.81983E-04
17	VELY	7204	A	1	05:001:00000	m/y	2	-0.102511002663171E-02	0.20579E-03
18	VELZ	7204	A	1	05:001:00000	m/y	2	0.274104971026836E-02	0.17380E-03
19	STAX	7216	A	1	05:001:00000	m	2	-0.132421109976204E+07	0.97620E-03
20	STAY	7216	A	1	05:001:00000	m	2	-0.533202313706689E+07	0.15257E-02
21	STAZ	7216	A	1	05:001:00000	m	2	0.323211831880121E+07	0.11575E-02
22	VELX	7216	A	1	05:001:00000	m/y	2	-0.126619959048342E-01	0.51526E-04
23	VELY	7216	A	1	05:001:00000	m/y	2	0.339720116768070E-03	0.84921E-04
24	VELZ	7216	A	1	05:001:00000	m/y	2	-0.467559096977887E-02	0.69110E-04
25	STAX	7213	A	1	05:001:00000	m	2	0.337060591485963E+07	0.65329E-03
26	STAY	7213	A	1	05:001:00000	m	2	0.711917602784800E+06	0.54467E-03
27	STAZ	7213	A	1	05:001:00000	m	2	0.534983082091565E+07	0.69138E-03
28	VELX	7213	A	1	05:001:00000	m/y	2	-0.142016652825457E-01	0.54415E-04
29	VELY	7213	A	1	05:001:00000	m/y	2	0.145280633229021E-01	0.36261E-04
30	VELZ	7213	A	1	05:001:00000	m/y	2	0.103568368182873E-01	0.76909E-04
31	STAX	7203	A	1	05:001:00000	m	2	0.403394735308593E+07	0.30487E-02
32	STAY	7203	A	1	05:001:00000	m	2	0.486990645775994E+06	0.11888E-02
33	STAZ	7203	A	1	05:001:00000	m	2	0.490043088922310E+07	0.34356E-02
34	VELX	7203	A	1	05:001:00000	m/y	2	-0.137947899098904E-01	0.23546E-03



Transformation between different ITRF

$$\begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ITRF_{yy}} = \begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ITRF\ 2000} + \begin{bmatrix} T_1 \\ T_2 \\ T_3 \end{bmatrix} + \begin{bmatrix} \delta & -\omega_Z & \omega_Y \\ \omega_Z & \delta & -\omega_X \\ -\omega_Y & \omega_X & \delta \end{bmatrix} \begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{ITRF\ 2000}$$

$$T_1(t) = T_1(t_0) + \left(\frac{dT_1}{dt}\right) \cdot (t - t_0)$$



Computation of Coordinates in EUREF 89

- From coordinates in ITRFyy for epoch t_0
 - Compute ITRFyy coordinates at 1989.0
 - Transform into EUREF 89 coordinates
 - Compute velocity in EUREF 89
- From GPS measurements and fiducial stations in ITRFyy
 - Transform ITRFyy coordinates of fiducial stations referred to epoch t_0 into coordinates referred to central measurement epoch t_c
 - Carry out field measurements at epoch t_c
 - Process GPS measurements to obtain ITRFyy coordinates referred to epoch t_c
 - Convert to EUREF 89 coordinates at epoch t_c
 - Convert to EUREF 89 coordinates at epoch 89.0



Reference systems of GNSS (=ITRS)

- GPS
 - WGS 84, defined in 1987 using TRANSIT observations. **1-2 m**
 - WGS 84 (G730) defined in 1994, WGS 84 (G873) defined in 1996, agrees with **ITRF94** at **10cm** level
 - WGS 84 (G1150), defined in 2002, agrees with **ITRF2005** up to **1cm**
- GLONASS
 - PZ-90, defined in 1990 using 26 ground stations
 - Agrees with ITRF at **meter level** (from a joint campaign in 1999)
 - PZ-90.11 realized in 2013 is compatible with ITRF 2008
- Galileo
 - GTRF agrees with ITRF within **1.5cm**



Some remarks

- Achievable accuracy of latest ITRF: **1cm, 1ppb**
- Facilitating transformation from national, triangulation-based coordinate systems to global geocentric GRS.
- Accurate geoid models to solve the problem:

$$\begin{array}{rcccl} 2 & + & 1 & \neq & 3 \\ (\varphi, \lambda) & + & (H) & \neq & (x, y, z) \end{array}$$



GRS overview

- Why? What is new?
- Astrogeodetic triangulation (2D)
- Height systems from levelling (1D)
- 3D geocentric reference systems
- Swedish geodetic infrastructure



Swedish Geodetic Infrastructure

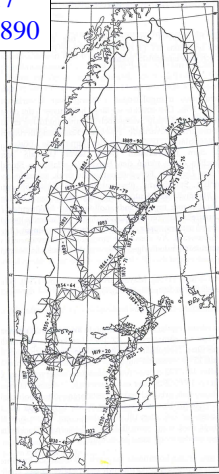


- Triangulation (2D) - RT90
- Precise levelling (1D) - RH 2000
- GNSS (3D) - SWEREF99
- Gravity - RG 82
- Geoid - SWEN 08LR
- Land uplift - RH 2000LU

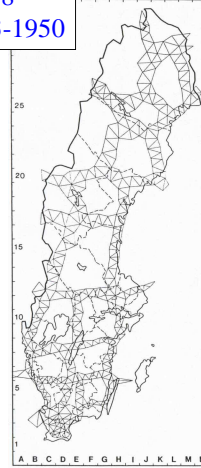


National 1st-order Triangulation

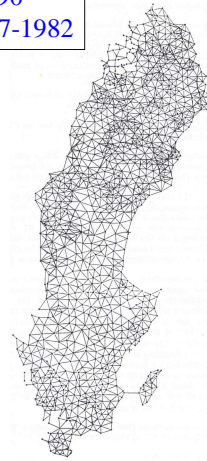
RT 1817
1815-1890



RT 38
1903-1950

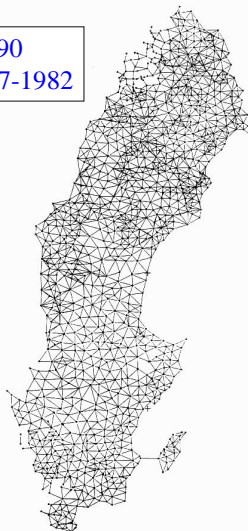


RT 90
1967-1982



3rd National Triangulation (RT 90)

RT 90
1967-1982



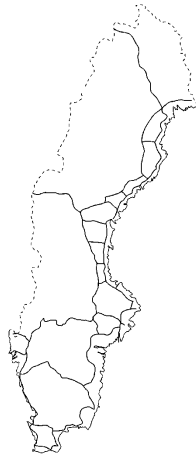
- Complete network with 15295 distances measured by **Geodimeters**®, 5424 angle measurements, 366 first-order points/baselines/angles of RT 38,



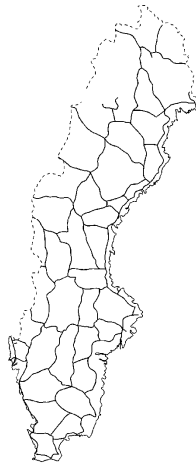


Swedish national levelling networks

RH 00
1886-1905



RH 70
1951-1967



RH 2000
1978-2003



Swedish Motorized Levelling





3rd Swedish Precise Levelling (RH 2000)



- 50000 benchmarks, ~ 1 benchmark per km
- Field precision: $< 1 \text{ mm}/\sqrt{\text{km}}$
- Motorized levelling technology used
- New height reference system: **RH 2000**
- Consistent with **EVRS 2000**:
 - Normal height is used
 - Zero height at Amsterdam
 - Zero tidal system (i.e. Permanent deformation is retained, while permanent tidal effect is removed)



Swedish 3D reference frame



- **SWEPOS**
 - 21 permanent GPS stations (25 since March 2000)
 - Realization of Swedish 3D reference system (ETRF89)
 - Monitoring GNSS system integrity
 - Support network-RTK – VRS (70km, 1-2cm (H), 2-3cm (V))
- **SWEREF 93**
 - 4 Swedish GPS stations in EUREF 89 campaign
 - 23 GPS stations in DOSE campaign, computed in ITRF 91 at epoch 1993.6
 - 11 DOSE points (5 Swedish) are fitted to EUREF coordinates
 - The final set of coordinates is named **SWEREF 93**



3D reference frame SWEREF 99



- Based on a Nordic GPS campaign in 1999, with 49 stations incl. 25 SWEPOS stations
- Coordinates computed in ITRF97 for 1999.5
- Transformed to EUREF89 for 1989.0
- Adopted in 2000 as an realisation of ETRS 89
- Introduced in 2001 as Swedish national reference frame **SWEREF 99**
- Tectonic epoch: 1989.0
- Height epoch: 1999.5 (GPS campaign)
- Ellipsoid: GRS 1980



Transform SWEREF 99 to RT 90/RH 2000

$$\begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{RT\ 90} = \begin{bmatrix} \delta x \\ \delta y \\ \delta z \end{bmatrix} + (1 + \delta s) \cdot R(\alpha_1, \alpha_2, \alpha_3) \cdot \begin{bmatrix} X \\ Y \\ Z \end{bmatrix}_{SWEREF\ 99}$$



$$\begin{aligned} \delta x &= -414.0979 \text{ metres} \\ \delta y &= -41.3381 \text{ metres} \\ \delta z &= -603.0627 \text{ metres} \\ \delta s &= +0.000\ 000\ 000\ 0 \cdot 10^{-6} \\ \alpha_1 &= -0.855\ 043\ 431\ 6'' \\ \alpha_2 &= +2.141\ 346\ 518\ 5'' \\ \alpha_3 &= -7.022\ 720\ 951\ 6'' \end{aligned}$$



Helmert model of 3D transformation

$$\begin{bmatrix} X_i^{(2)} \\ Y_i^{(2)} \\ Z_i^{(2)} \end{bmatrix} = \begin{bmatrix} \delta x \\ \delta y \\ \delta z \end{bmatrix} + s \cdot R(\alpha_1, \alpha_2, \alpha_3) \cdot \begin{bmatrix} X_i^{(1)} \\ Y_i^{(1)} \\ Z_i^{(1)} \end{bmatrix}$$

$$s = 1 + \delta s$$

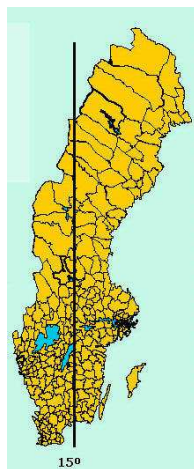
$$R_{3 \times 3} = R(\alpha_1, \alpha_2, \alpha_3) = R_3(\alpha_3) \cdot R_2(\alpha_2) \cdot R_1(\alpha_1)$$

$$= \begin{bmatrix} \cos \alpha_3 & \sin \alpha_3 & 0 \\ -\sin \alpha_3 & \cos \alpha_3 & 0 \\ 0 & 0 & 1 \end{bmatrix} \cdot \begin{bmatrix} \cos \alpha_2 & 0 & -\sin \alpha_2 \\ 0 & 1 & 0 \\ \sin \alpha_2 & 0 & \cos \alpha_2 \end{bmatrix} \cdot \begin{bmatrix} 1 & 0 & 0 \\ 0 & \cos \alpha_1 & \sin \alpha_1 \\ 0 & -\sin \alpha_1 & \cos \alpha_1 \end{bmatrix}$$

$$= \begin{bmatrix} \cos \alpha_2 \cos \alpha_3 & \cos \alpha_1 \sin \alpha_3 + \sin \alpha_1 \sin \alpha_2 \cos \alpha_3 & \sin \alpha_1 \sin \alpha_3 - \cos \alpha_1 \sin \alpha_2 \cos \alpha_3 \\ -\cos \alpha_2 \sin \alpha_3 & \cos \alpha_1 \cos \alpha_3 - \sin \alpha_1 \sin \alpha_2 \sin \alpha_3 & \sin \alpha_1 \cos \alpha_3 + \cos \alpha_1 \sin \alpha_2 \sin \alpha_3 \\ \sin \alpha_2 & -\sin \alpha_1 \cos \alpha_2 & \cos \alpha_1 \cos \alpha_2 \end{bmatrix}$$

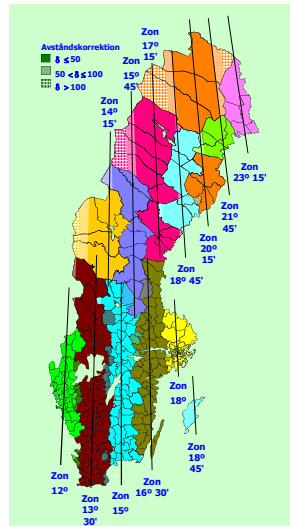


National Projection - SWEREF 99 TM



- (x, y, z) -coordinates based on SWEREF 99
- Geodetic coordinates (φ, λ, h) referred to GRS 80 reference ellipsoid
- Planar coordinates (x, y) in Gauss conformal projection, i.e Transversal Mercator projection
- Central meridian $\lambda_0 = 15^\circ$
- Scale reduction factor: $k_0 = 0.9996$
- False easting: y-coordinates shifted by +500 km
- Whole Sweden in one UTM zone (UTM 33)

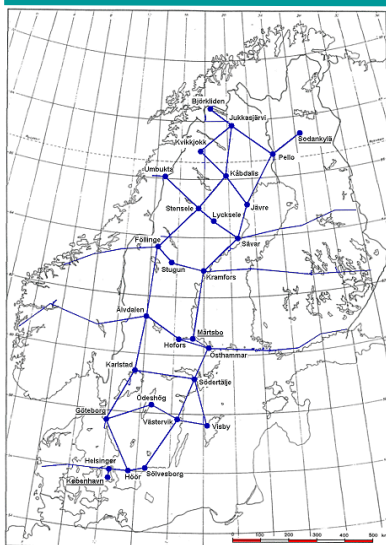
Regional projection zones



- 12 zones: **SWEREF 99 dd mm**
- 12 Central meridians:

12° 00'	14° 15'
13° 30'	15° 45'
15° 00'	17° 15'
16° 30'	18° 45'
18° 00'	20° 15'
	21° 45'
	23° 15'
- Scale reduction factor: $k_0 = 1$
- False easting: $y + 150 \text{ km}$

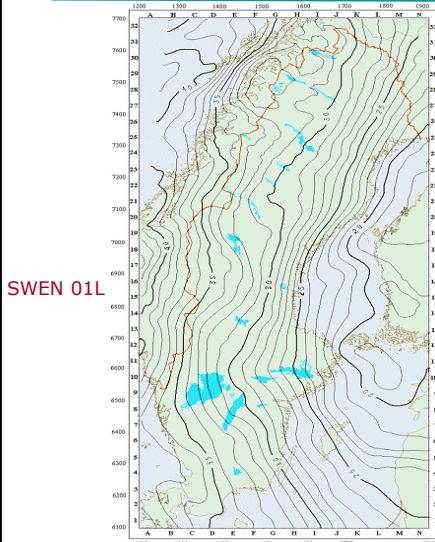
The 3rd zero-order gravity network - RG82



- RG 41, RG 62, RG 82
- 25 zero-order points measured 1981-82
- Relative gravity measurements, connected to other Nordic countries
- 4 absolute gravity points:
 - Mårtsbo, Göteborg, Copenhagen and Sodankylä
- Mårtsbo: **$g = 981\,923,484 \text{ mgal}$**
- Time epoch: 1982.0
- Zero tidal system



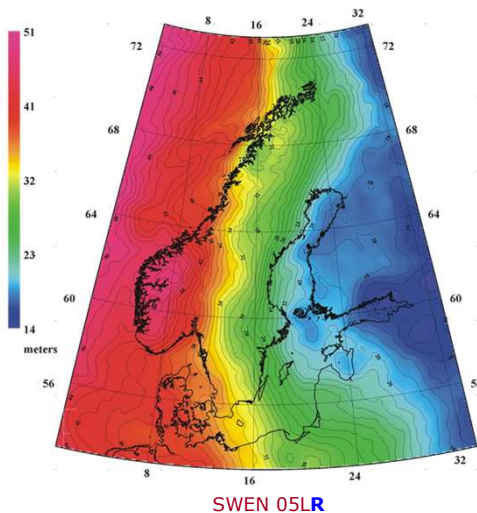
Swedish geoid models



- **RN 92**
 - NKG 89 geoid computed at KMS, using OSU89 and GRS 80
 - NKG 89 transformed to refer to RT 90
- **SWEN 01L**
 - NKG 96 computed using EGM 96
 - NKG 96 fitted to SWEREF 99 and RH 70 at 91 GPS/levelling points (4 cm rms, 15cm max. residuals)
 - Land uplift between 1970.0 and 1999.5 has been included
 - A height correction model



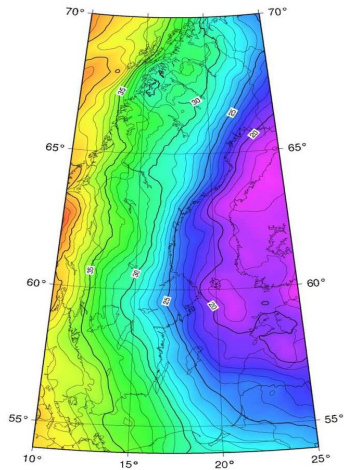
Gravimetric Geoid Model - SWEN 05LR



- NKG 2004 fitted to SWEREF 99 and RH 2000 at 1178 GPS/levelling points
- 0.5 years land uplift correction (between 1999.5 and 2000.0)
- Remaining residuals are smoothly interpolated
- A height transformation model from SWEREF 99 to RH 2000
- Accuracy:
 - 1.3 cm at "RIX 95" GPS points
 - 3.7 cm other places
 - 5-10 cm in mountain areas



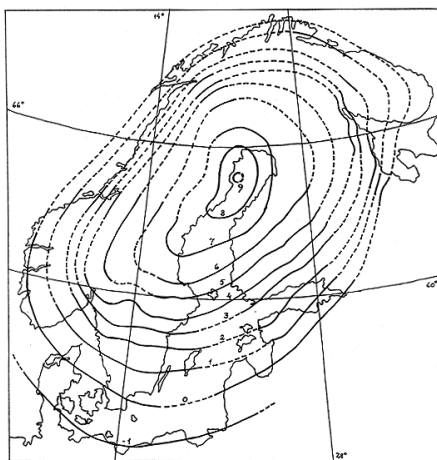
Gravimetric Geoid Model - SWEN 08LR



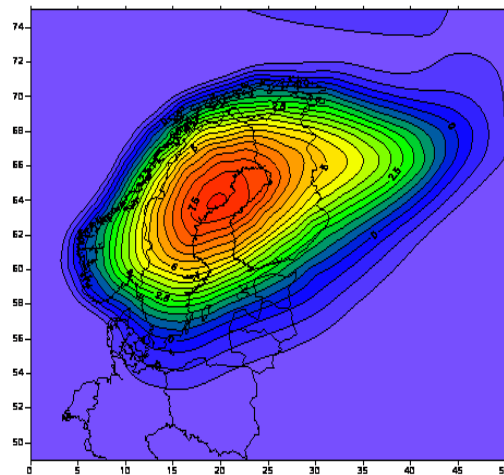
- Computed using the KTH method by modifying Stokes' formula and combining global gravitational models with terrestrial gravity data.
- Fitted to 1570 GPS/levelling points (SWEREF 99 / RH 2000)
- Remaining residuals are smoothly interpolated
- A height transformation model from SWEREF 99 to RH 2000
- Accuracy:
 - 1 - 1.5 cm in most areas
 - 5 - 10 cm in Northwest without levelling
- **SWEN 15LR** to be introduced soon.



Swedish land uplift model (mm/year)



Martin Ekman (1998)



RH 2000 LU